

STATE OF LOUISIANA DEPARTMENT OF TRANSPORTATION AND DEVELOPMENT OFFICE OF PUBLIC WORKS



WATER RESOURCES
TECHNICAL REPORT
NO. 39

PROJECTED EFFECTS OF PROPOSED INCREASED PUMPAGE
ON WATER LEVELS AND SALINITY IN THE
SPARTA AQUIFER NEAR WEST MONROE, LOUISIANA

Prepared by
UNITED STATES DEPARTMENT OF THE INTERIOR
GEOLOGICAL SURVEY

In cooperation with
U. S. Army Corps of Engineers
Vicksburg District
and

LOUISIANA DEPARTMENT OF TRANSPORTATION AND DEVELOPMENT OFFICE OF PUBLIC WORKS

1985

UNITED STATES ARMY CORPS OF ENGINEERS VICKSBURG DISTRICT

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WEST MONROE, LOUISIANA

Ву

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U.S. Geological Survey

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FACTORS FOR CONVERTING INCH-POUND UNITS TO INTERNATIONAL SYSTEM (SI) OF UNITS

Multiply	<u>By</u>	To obtain
cubic foot per second (ft ³ /s)	0.02832	cubic meter per second (m ³ /s)
foot (ft)	0.3048	meter (m)
foot per day (ft/d)	0.3048	meter per day (m/d)
foot per year (ft/year)	0.3048	meter per year (m/year)
square foot per day (ft2/d)	0.9290	square meter per day (m ² /d)
mile (mi)	1.609	kilometer (km)
square mile (mi ²)	2.590	square kilometer (km²)

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PROJECTED EFFECTS OF PROPOSED INCREASED PUMPAGE ON WATER LEVELS AND SALINITY IN THE SPARTA AQUIFER NEAR WEST MONROE, LOUISIANA

By Douglas A. Trudeau and Anthony Buono

ABSTRACT

A ground-water model of the Sparta aquifer was developed to evaluate the effects on water levels and salinity of proposed increased pumping to meet projected water-supply requirements for the Monroe-West Monroe area, La. Estimates of projected water-supply requirements were made by the U.S. Army Corps of Engineers. The area of study centered on West Monroe and encompassed the area of occurrence of the Sparta sand from its western outcrop area in Louisiana and southern Arkansas to its eastern outcrop in Mississippi, an area of approximately 66,000 square miles. A detailed three-dimensional model of the aquifer and overlying and underlying confining layers and aquifers was prepared and calibrated for the period 1900-1964, using parameter-estimation techniques and the observed change in water levels in wells in the Sparta aquifer. The model indicates that 45 percent of the total water pumped from the Sparta aquifer between 1900 and 1980 was derived from leakage through confining layers, 38 percent from recharge in the outcrop-subcrop area, and 17 percent was released from storage in the aquifer.

Eight alternate pumping plans proposed by the Corps of Engineers for the period 1980-2040 were evaluated using the developed model. These plans projected additional pumpage from the Sparta aquifer ranging from 10 to 226 cubic feet per second. Water-level declines simulated by the model indicate that water levels in wells in the Sparta aquifer will eventually drop below the top of the aquifer near West Monroe with all plans. maximum projected water-level decline for the various plans ranges from 51 to 296 feet. Evaluation of the effects of the various pumping plans on the rate of movement of downdip saltwater towards West Monroe indicates that the rate would increase 34 to 74 percent from the 1980 projected rate of about 30 to 100 feet per year; however, this would not result in a significant change in water quality at West Monroe for at least 200 years. Pumping plan A, which calls for an increase in pumpage of 10.58 cubic feet per second through 2040, would minimize the areal extent of excessive drawdown. In addition, it would produce the slowest rate of water movement from the saltwater front towards West Monroe.

INTRODUCTION

Background and Location of the Study Area

The Sparta Sand is the most important aquifer in northern Louisiana and is also an important source of water in southern Arkansas and western Mississippi. Potentiometric levels of the Sparta aquifer have been declining significantly since 1920 in response to the development of major pumping centers in Arkansas and Louisiana. Major pumping centers currently are located near West Monroe, Jonesboro-Hodge, and Bastrop, La.; near Magnolia and El Dorado, Ark.; and near Jackson, Miss. Water-level declines caused by these pumping centers have formed overlapping cones of depression. The cone of depression at West Monroe in northeastern Louisiana is the center of interest for this study. To evaluate this cone, the area of study encompassed the area underlain by the Sparta Sand from its western outcrop in Louisiana and southern Arkansas to its eastern outcrop in Mississippi. This area is 210 mi wide by more than 315 mi long. (See fig. 1.)

Purpose

The U.S. Army Corps of Engineers, as part of a study of the Ouachita River basin, has projected the water use for the Monroe-West Monroe area in Ouachita Parish, La., for the period 1980-2040. The purpose of this study was to estimate the effects on water levels and salinity of the Sparta aquifer if the projected pumpage were obtained from the Sparta. Proposed well fields would be installed near West Monroe.

Approach

A ground-water model of the aquifer and related confining beds and underlying and overlying aquifers was developed to determine the impact of the proposed pumpage from the Sparta aquifer near West Monroe. Development of the model involved first determining whether the aquifer could be treated as a single hydrologic unit, as in earlier modeling studies, or as one of a series of hydraulically connected aquifers. To determine whether the Sparta aquifer is hydraulically connected to other aquifers, preliminary two- and three-dimensional models were prepared before developing the detailed model. Data to define the hydrogeologic properties of confining layers and aquifers for the detailed model were gathered from the The model was prepared to simulate historical declines in water levels and was calibrated against records of the decline from 1900 This calibration was confirmed by model simulations of through 1964. water-level declines from 1964 through 1979. The model was then used to project the impact of the proposed increased pumpage near West Monroe on water levels, 1980-2040.

Increased pumpage from the Sparta aquifer will possibly increase the rate of movement of a saltwater front in the aquifer that is moving slowly towards West Monroe. To determine if the proposed increased pumpage will cause changes in the water quality at West Monroe, water levels projected by the model were used to estimate the change in rate of movement of the front.

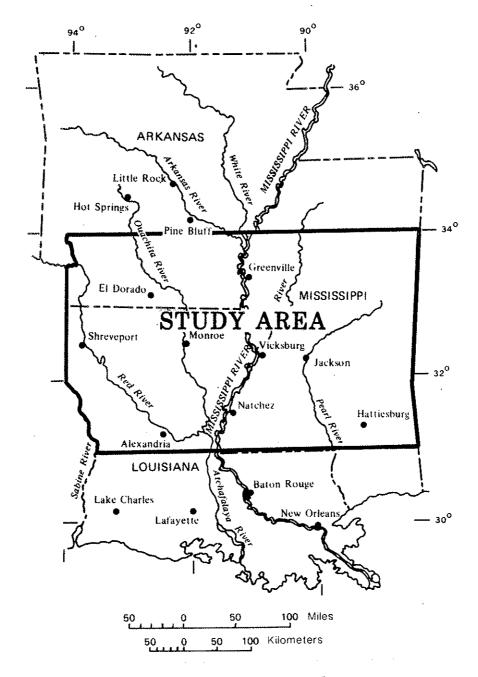


Figure 1.--Location of the study area.

Previous Investigations

Data from the reports of Payne (1968, 1970, 1972, and 1975), Hosman and others (1968), Ryals (1980a, 1980b, 1982, and 1984), and Rogers and others (1972) were used to help define the aquifer systems for modeling. Hydrologic data for the Cockfield Formation, Sparta Sand, Cane River Formation, and Carrizo Sand in the study area were obtained from the reports of Payne (1968, 1970, 1972, and 1975). Additional data for the

Tertiary aquifers in the Mississippi embayment were obtained from a report by Hosman and others (1968). Data from reports by Ryals (1983a, 1983b, and 1984) were used to describe the thickness of the Cook Mountain Formation and Carrizo-Wilcox aquifer and the hydraulic characteristics of the Carrizo-Wilcox aquifer. Data on the ground-water resources in the Monroe-West Monroe area were obtained from a report on Ouachita Parish by Rogers and others (1972).

Previous efforts to model the Sparta aquifer include an analog model of the Sparta in the Mississippi embayment by Reed (1972) and digital modeling of the Sparta in the Ruston area by Sanford (1973). Information from these models was used in preliminary modeling and in development of the detailed three-dimensional model.

Well-Numbering System

In Louisiana, the U.S. Geological Survey assigns water wells a number in approximate order of inventory; a two letter prefix represents the parish in which the well is located. For example, Ou-1 is the first well inventoried in Ouachita Parish. Where more than one test well has been developed consecutively in the same test hole, the different wells are indicated by a letter suffix. For example, wells Ou-71A and Ou-71B are test wells installed in test hole Ou-71.

GEOHYDROLOGY

General Geohydrologic Framework

Geologic units at West Monroe and in the study area were classified as aquifers or confining layers for preparing a ground-water model. The discussion of the general geohydrologic framework in the study area and around West Monroe as it relates to modeling the Sparta aquifer follows.

The West Monroe area is on the western limb, of the Mississippi embayment, "...a geosyncline plunging gently to the south, the axis of which roughly follows the present course of the Mississippi River." (Cushing and others, 1970, p. Al). The stratigraphic units of interest as aquifers or confining layers (table 1) are from oldest to youngest: the Wilcox group, the Carrizo Sand, the Cane River Formation, the Sparta Sand, the Cook Mountain Formation, the Cockfield Formation and undifferentiated upper Tertiary deposits. The general outcrop and sub-surface configuration of the units are shown in figures 2 and 3, respectively. They show the general nature of the Mississippi embayment in the study area with units cropping out on both sides of the axis of the embayment and thickening toward the center of the embayment.

The following paragraphs give a general description of the aquifers and confining layers. For a more detailed description of the aquifers and confining layers in the study area, refer to the reports mentioned in the Previous Investigations section of this report.

Table 1.--Ceneralized description of aquifers in northern Louisiana

[Feet, ft; feet per day, ft/d; gallons per minute, gal/min]

Hydrologic characteristics	Contains freshwater. Used locally for rural supplies and some public supplies. Yields range from a few gal/min for small domestic supplies to several thousand gal/min for large irrigation wells. Hydraulic conductivity ranges from 100 to 300 ft/d.	Contains freshwater and saltwater. Hydraulic conductivity ranges from about 25 ft/d to more than 100 ft/d.	Generally not water bearing. Local sands yield small quantities of water to wells.		Contains freshwater and saltwater. Used mostly for small rural supplies. Hydraulic conductivity ranges from less than 15 ft/d to more than 40 ft/d.	Generally not water bearing. Local sands yield small quantities of water to wells.	Contains freshwater and saltwater. Principal aquifer of north-central Louisiana. Large withdrawals by domestic, municipal, and industrial wells. Hydraulic conductivity ranges from 30 ft/d to more than 100 ft/d.	Not water bearing.	Contains freshwater and saltwater. Used only by a few shallow wells, mostly in the outcrop. Hydraulic conductivity about 25 ft/d.	Contains freshwater and saltwater. Used mostly by small- yielding rural wells. Larger supplies developed locally where sands are thick. Hydraulic conductivity about 15 ft/d.
Aqui- fer	Cuaternary aquifers	Miccene aquifers			Cockfield aguifer		Sparta aquifer		rter o-wilcox	
Description	Terrace remmants alluvial valley fill. Coarse, graveliferous at base grading upward to sand, silt, and clay. Thickness about 50 to 150 ft.	Interbedded sand and clay. Thickness 400 to 800 ft.	Mostly clay. Thickness 400		Fine lignitic sand and carbon- aceous clay. Thicker sands in lower part. Thickness about 500 to 600 ft.	Clay, partly sandy and glauconitic. Thickness about 100 to 200 ft.	Fine to medium sand with clay interbeds; lignitic. Thick- ness 500 to 700 ft.	Mostly clay; some marl. Thick- ness 200 to 300 ft.	Fine to medium sand; discontinuous. Thickness 0 to 150 ft.	Interbedded sand, clay, and silt, lignitic. Thickness 500 to 1,500 ft.
Forma- tion		-ibnU bebiv	Undivided	pəbivibnU	Cockfield Formation	Acco Moun. Tain Tim.	strage base	Cane River Fm.	Carrizo Sand	papivibnu (
Group		−ibnU b⊕biv	Vicksburg	ıgckson			эшк	Claibo		Мі.1сок
Series (Holoœne and Pleistocene	Miccene	Offdocene				Eocene		CALVIS ENVEYOR	Paleo-
Sys- tem	Gnaternary	Jertiary Quaternary								
Bra	Cenzoic									

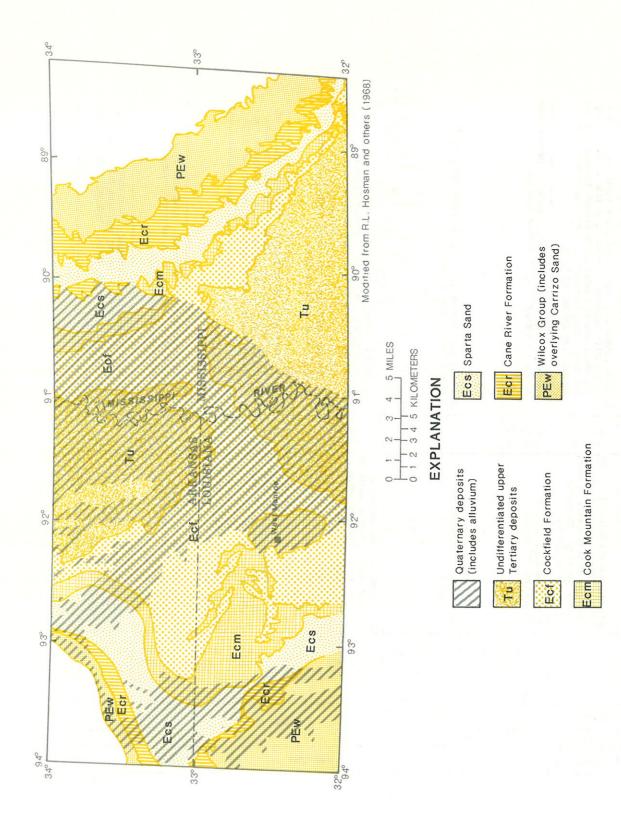


Figure 2.--Generalized geologic map showing the geologic units of interest in the study area.

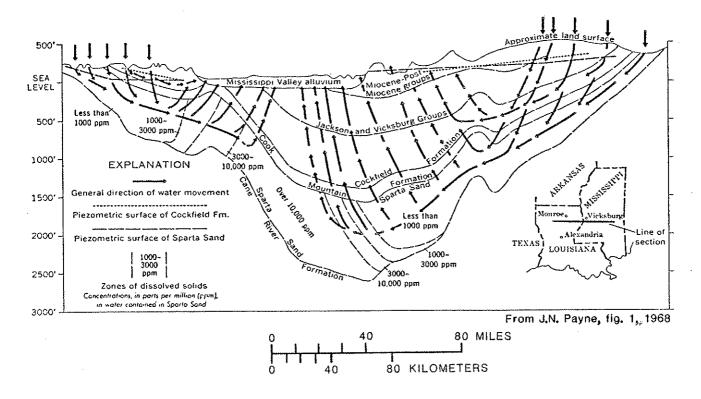


Figure 3.--Geohydrologic section from western outcrop to eastern outcrop of the Sparta Sand.

The Wilcox Group and the Carrizo Sand are treated as a single hydrologic unit, the Carrizo-Wilcox aquifer, because the Wilcox has been shown by Ryals (1982) to be hydraulically connected to the overlying Carrizo. The Carrizo-Wilcox consists of fine to medium sand and interbedded clay and silt ranging from 400 to 1,500 ft thick.

The Cane River Formation is the confining layer that overlies the Carrizo-Wilcox aquifer and separates the Carrizo-Wilcox from the Sparta Sand. The formation consists mainly of clay and silty clay and ranges in thickness from 200 to more than 300 ft.

The Sparta aquifer, the principal aquifer in the study, consists of interbedded sand, silt, and clay ranging from 500 to 700 ft in thickness. Regionally, the sands are hydraulically connected from top to bottom and respond as one unit. In Ouachita Parish, the area of specific interest, the base of the unit ranges from 650 to 850 ft below sea level and ranges from 600 to 700 ft in thickness. The top of the Sparta is less than 100 ft below sea level in the western part of the parish and about 200 ft below sea level in the eastern part of the parish.

Overlying the Sparta aquifer is the Cook Mountain Formation, a confining bed. Although the unit is predominantly clay, thin sands make it a local source of freshwater. The Cook Mountain crops out in the northwestern part of the parish and ranges from less than 100 to more than 200 ft in thickness.

The Cockfield Formation, the uppermost geologic unit considered an aquifer in this system of aquifers, consists of interbedded sand and clay. The aquifer is missing in the Ouachita River valley (fig. 2) because of erosion by the ancestral Ouachita River and has been replaced by alluvial sands and gravel. In the Mississippi River valley, the Cockfield is overlain by Mississippi River alluvium. The Cockfield and the alluvium are hydraulically connected and act as a single hydrologic unit (fig. 3).

Overlying the Cockfield Formation in parts of the embayment (fig. 2) are undifferentiated deposits (predominantly clay) of Tertiary age that range from 400 to 700 ft in thickness and act as a confining zone.

The relations of recharge, discharge, and regional flow in the unstressed, natural-flow system to the geology are shown in figure 3. Figure 3 does not show the Cane River Formation or Carrizo-Wilcox aquifer. The relations of recharge, discharge, and regional flow for these units are believed to be similar to those shown in figure 3 for the other units. The Sparta Sand and the other aquifers in this study are recharged in their outcrop areas directly by infiltration of rainfall and indirectly in the subcrop area by downward percolation through surficial materials. A large part of the water that infiltrates to the aquifers is returned as base flow to the streams in the outcrop area. Only a small part of the water that infiltrates enters the deep percolation system of the aquifers. Water that enters the Cockfield, Sparta, and Carrizo-Wilcox aquifers as recharge is confined a short distance downdip by clay beds within the respective formations or by overlying clay. Discharges of this water are primarily to the Mississippi River alluvial valley by upward leakage through overlying aquifers and confining beds (fig. 3). Development of the aquifers for water supplies has materially altered the natural-flow system shown in figure 3.

Pumpage and Decline in Potentiometric Level

A comparison of potentiometric maps of the Sparta aquifer for 1886 (fig. 4), prior to extensive development of the aquifer, and for 1980 (fig. 5) shows that several major pumping depressions have formed in the potentiometric surface of the aquifer. These depressions have developed in response to major pumping centers that are located near Bastrop, Jonesboro-Hodge, and West Monroe, La.; El Dorado and Magnolia, Ark.; and Jackson, Miss. In addition, another major pumping depression has developed in the vicinity of Pine Bluff, Ark., located just north of the study area.

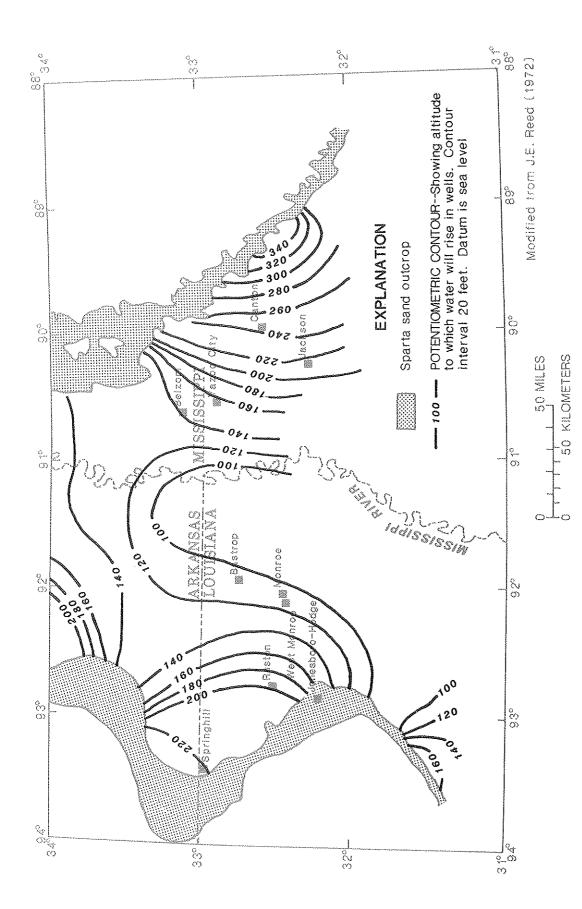
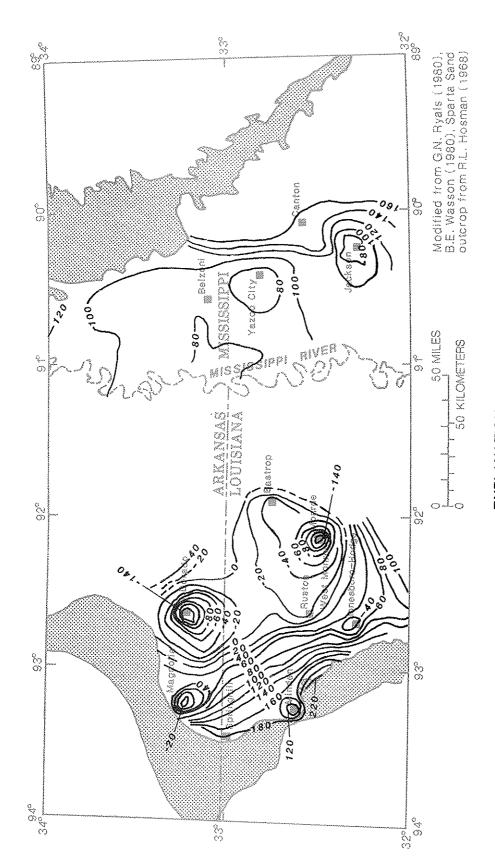


Figure 4.--Potentiometric surface of the Sparta Sand, 1886.



EXPLANATION

Sparta Sand outcrop

POTENTIOMETRIC CONTOUR -- Showing altitude to which water will rise in wells. Contour interval 20 feet. Dashed where approximate. Datum is sea level 1 -100-

Figure 5. -- Potentiometric surface of the Sparta Sand, 1980.

Records of water use from the Sparta aquifer prior to 1960 are intermittent to nonexistant. Sanford (1973) reported an average pumping rate from the major pumping centers in Arkansas and Louisiana for the period 1925-65. Sanford concluded that, although development of the aquifer began around 1900, significant water-level declines began to occur only after 1920. Since 1960, the U.S. Geological Survey has been gathering data on water use, and more complete records are available. Table 2 shows the pumpage rates at major centers of water use in Arkansas, Louisiana, and Mississippi. Pumpage from the Sparta aquifer at the major pumping centers during 1975 was 201 ft³/s; total pumpage was 239 ft³/s.

Few data are available on the impact of pumpage on potentiometric levels of the Cockfield and Carrizo-Wilcox aquifers in the study area. Data are available only for areas where the aquifers contain freshwater, which are near the outcrop areas of these formations and represent only a small part of the modeled area.

As shown in figure 2, the Cockfield underlies shallow Quaternary deposits in much of the study area. Water use in the study area from the Cockfield during 1975 totaled about 45 $\rm ft^3/s$. No significant pumping depressions occur in the Cockfield in Arkansas and Louisiana because there are no major pumping centers. In Mississippi, however, several significant pumping depressions occur associated with the large pumping centers (Wasson, 1981b).

Table 2.--Pumpage at major pumping centers from the Sparta aquifer, 1925-80

[Data for Arkansas, 1965-80, from unpublished data furnished by Arkansas District, U.S. Geological Survey. Data for Louisiana, 1965-80, from unpublished data on file in Louisiana District, U.S. Geological Survey. In cubic feet per second]

	1925-65	1965	1970	1975	1980
Pine Bluff, Ark	a _{20.39}	63.85	90.08	71.01	
Magnolia, Ark	b4.64	1.89	5 . 75	4.93	
El Dorado, Ark	b24.75	27.00	24.05	18.43	
Bastrop, La	b15.47	18.24	17.43	18.46	16.49
Hodge, La	b _{15.47}	20.50	20.50	21.74	20.87
Monroe, La	b _{20.11}	22.97	15.80	20.57	22.95
Ruston, La	b2.48	5.11	4.81	6.28	7.37
Springhill, La	b10.51	16.70	9.90	7.78	.90
Yazoo City area, Miss				C14.07	
Belzoni, Miss				cl.67	
Jackson area, Miss	d6.68	e15,56		c12.99	
Canton, Miss		e _{1.16}		¢3.09	,

a From Bedinger and others (1960).

b From Sanford (1973).

C From Newcome (1976).

d From Harvey and others (1964).

e From Shattles and others (1967).

Water use from the Carrizo-Wilcox aquifer in Arkansas and Louisiana and its equivalents in Mississippi, during 1975, totaled about 65 $\rm ft^3/s$. The aquifer has not been extensively developed in Arkansas and Louisiana; therefore, no significant pumping depressions occur. In Mississippi, there are several cones of depression associated with major pumping centers (Wasson, 1980).

Evidence to show hydraulic connection between the Carrizo-Wilcox, the Sparta, and the Cockfield aquifers in areas where the Sparta is heavily pumped is sparse. Water levels in sands in the Cook Mountain Formation, overlying the Sparta, have declined from 5 ft to as much as 40 ft near Ruston and near Monroe, La. (Sanford, 1973; Rogers and others, 1972) in response to heavy pumping from the Sparta. This suggests hydraulic connection between the Cook Mountain confining clay and the Sparta, but no downward trend in water levels in wells in the Cockfield aquifer has been observed. Information on gradients from the Carrizo-Wilcox aquifer and Cane River confining clay to the Sparta near large pumping centers could not be obtained.

The approach used to determine the effects of proposed increased pumpage near West Monroe on water levels was to use a model that simulated historic declines in water level in wells in the Sparta aquifer.

Water Quality

The dissolved-solids concentration of ground water as an indication of salinity is the only water-quality parameter used in this study. The existing patterns of water salinity must be considered in evaluating effects of the proposed increased pumpage from the Sparta aquifer on water quality at West Monroe. As the Sparta may be hydraulically connected to aquifers above and below it, the potential effects of influx of water from these aquifers must also be considered.

Plate 1 (after Payne, 1968, 1970, and 1975) shows the downdip limit of freshwater in the Cockfield Formation, Sparta Sand, Carrizo Sand, and equivalents. For mapping purposes, Payne defined freshwater as water having a dissolved-solids concentration of 1,000 mg/L or less. Plate 1 indicates that near West Monroe, the Cockfield and the Sparta contain freshwater, but the water becomes saline to the southeast, and the Carrizo-Wilcox contains saline water. Because water in the Cockfield is fresh near West Monroe, inflow of water from that aquifer poses no water-quality problem to the Sparta aquifer at West Monroe. Potentiometric contours in figure 5 indicate that ground-water movement in the Sparta is from the saline front, which lies to the south towards West Monroe. The proposed pumpage increases near West Monroe would affect the movement of the saline water towards West Monroe by lowering potentiometric levels and increasing the northward gradient. The underlying Carrizo-Wilcox aquifer contains saline water at West Monroe and by leaking upward, also may pose a threat to the water quality of the Sparta. However, because of the lack of data on water levels in the Carrizo-Wilcox aquifer, no reasonable estimate of water-movement rates can be made.

DEVELOPMENT OF A THREE-DIMENSIONAL MODEL

To determine the impact on water levels of the Sparta aquifer that might be caused by proposed increases in pumpage near West Monroe, a digital model of the Sparta aquifer was prepared. The approach used in modeling the aquifer system was to develop a model that could simulate changes in water level (drawdown) from 1900 through 1964. It was assumed that in 1900 water levels were stable because there was little or no pumping. The model of the aquifer system was used to generate the stable (steady-state) water-level map of 1900. The steady-state water-level map then was used in combination with the computed change in water levels to prepare computed water-level maps. The model was further tested on how well it could simulate change in water levels between 1965 and 1979. Once the model passed this final testing, it was used to analyze the effects on water levels of projected increased pumpage from the Sparta aquifer.

Finite-difference models developed by Trescott and others (1976) and by McDonald and Harbaugh (1984) were used during this study. The Trescott model was used to simulate ground-water flow in two directions and to simulate vertical leakage from a river. The McDonald-Harbaugh model was used to simulate ground-water flow in three directions. The basic equations governing ground-water flow that these models solve by the finite-difference method are documented in the above-mentioned references. The strongly-implicit procedure was used to solve the equations of ground-water flow. (See above references.)

Preliminary two- and three-dimensional models of the Sparta aquifer system were prepared to determine whether the Sparta was the only aquifer to be included in the model. Both the Trescott and others (1976) and McDonald and Harbaugh (1984) models were used for these early studies. On the basis of concepts developed from the preliminary two- and threedimensional models, a detailed three-dimensional model was made of the aquifer system using the McDonald model. The modeled system is composed of (from top to bottom) the Cockfield aquifer, the Cook Mountain confining layer, the Sparta aquifer, the Cane River confining layer, and the Carrizo-Wilcox aquifer. Preliminary modeling of the aquifer system indicated that the undifferentiated upper Tertiary deposits did not need to be included in the model to simulate changes in the water level of the Sparta aquifer. The geohydrologic units of which the aquifer system is comprised and the way they were treated in the model are graphically illustrated in figure 6. Basically, each aquifer was modeled from outcrop area to outcrop area and treated as a confined aquifer. Each confining layer was treated as a zone through which vertical flow occurs.

It was decided to model each aquifer as a confined aquifer because this closely approximated the natural system and also minimized the model input requirements; the latter was important because of the short time allotted to this investigation. The assumption that the aquifers behave as confined aquifers is valid as long as dewatering (water levels declining below the bottom of the overlying confining layer) does not occur. Dewatering of the Sparta was not anticipated to be a major problem, considering the initial values of pumpage projected by the U.S. Army Corps of Engineers.

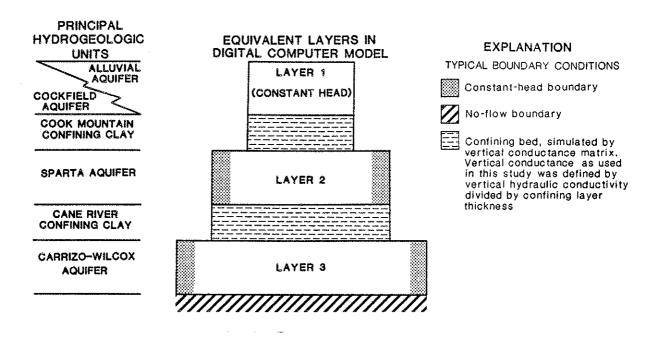


Figure 6.--Principal hydrogeologic units and equivalent layers in digital-computer model.

Model Construction

Finite-Difference Grid and Hydrogeologic Parameters

The principal area of interest was the area around West Monroe, where the proposed increased pumpage is to occur. The size of grid blocks around West Monroe, in Ouachita Parish, was chosen so that pumping centers and observation wells would be near grid nodes and to facilitate the analysis of the proposed pumping. Grid blocks in Ouachita Parish were 2 mi by 2 mi. The finite-difference grid used in the detailed model was oriented in a north-south, east-west direction to incorporate the major north to northeast axis of transmissivity reported by Payne (1968, p. A8) for the western limb of the Mississippi embayment. Although Ouachita Parish is the principal area of interest, the Sparta aquifer is strongly affected by pumpage in other areas. Accordingly, the grid was gradually expanded to cover the study area. The model grid, which consists of 26 rows and 35 columns, represents an area about 210 mi wide and about 315 mi long as shown on plate 2. The way in which the Sparta aquifer was modeled -- the model grid, the location and the type of model boundaries used, and the location of pumping centers, are also shown on plate 2.

The hydrogeologic parameters required as input to the detailed three-dimensional model in order to define each modeled layer (fig. 6) consisted of the following: transmissivity, storage coefficient, and vertical conductance of confining beds between layers. Note that the vertical conductance is defined as the vertical hydraulic conductivity divided by the thickness of the confining bed. Data on aquifer and confining-layer properties for the detailed three-dimensional model were obtained from previous investigations of the Tertiary aquifers in the Mississippi embayment (Hosman and others, 1968; Payne, 1968, 1970, 1972, and 1975) or were calculated during the preliminary modeling stage of this study. The values used to describe the geohydrologic units in the model are summarized in table 3.

Generally, input to the detailed model was obtained by overlaying the finite-difference grid on maps of hydrogeologic properties and interpolating the values for each node. The transmissivity of the Sparta and the thickness of confining layers were input to the model in this manner. Input to the model for the storage coefficient of the Sparta aquifer was obtained by plotting published data for storage coefficient on the model grid and estimating values over the rest of the grid. Input to the model for some hydrogeologic parameters were average values, which were assumed constant over the entire grid. Average values were used for the transmissivity and storage coefficient of both the Cockfield and Carrizo-Wilcox aquifers. Values of vertical hydraulic conductivity for determination of vertical conductance of confining layers were estimated values obtained from preliminary modeling.

Boundary Conditions

Model boundaries were chosen to closely approximate the natural hydrologic boundaries. The boundary conditions specified for the Sparta aquifer were no-flow on the northern and southern boundaries and constant-head on the western and eastern boundaries. The boundary conditions for the Carrizo-Wilcox aquifer were the same as those specified for the Sparta. The Cockfield aquifer was treated as a constant head layer. Justification for these boundary specifications follows.

The southern boundary of the Sparta aquifer also was treated as a no-flow boundary. The Sparta aquifer thins to the south and gradually grades to clay, producing a natural no-flow boundary. The exact location where the aquifer becomes no-flow is unknown. The southern boundary of the model was set at 31 degrees north latitude based on preliminary modeling results which showed no difference between the use of a no-flow or constant-head boundary at this location.

The northern boundary of the Sparta aquifer was treated as a no-flow boundary. Potentiometric-contours of the Sparta in the Mississippi embayment (Reed, 1972) show that a ground-water divide has developed between the El Dorado and Pine Bluff, Ark., pumping centers. The divide occurs approximately at 34 degrees north latitude. This ground-water divide acts as a no-flow boundary.

Table 3.--Hydrogeologic parameters used in the detailed three-dimensional model

Aquifer parameters							
Aquifer	Transmissivity (square foot per second)	Storage coefficient					
Cockfield	a _{0.068} c _{0.02} -0.25 d _{0.065}	b _{0.1} d _{0.1-0.0001} 0.1 in outcrop 0.001 elsewhere					
	Confining-layer parameters	·					
Confining layer	Vertical hydraulic conductivity (foot per second)	Thickness (feet)					
Cook Mountain Cane River	e1.46 X 10 ⁻¹⁰ e6.08 X 10 ⁻¹¹	£ ₅₀ -650 925-600					

a Average of values reported by Hosman and others (1968).

b Assumed.

9 Payne (1972), plate 1.

The western boundary of the Sparta aquifer is the western limit of the outcrop-subcrop area. The aquifer outcrop-subcrop area was treated as a constant head boundary to simulate additional recharge water available to pumping centers. Streams in the outcrop area are perennial with base flow sustained by discharging ground water. Until such time as sufficient water is captured by the drawdown cones so that streams dry up in the outcrop-subcrop area, use of constant-head nodes seems justified. The potentio-metric surface of the Sparta aquifer (fig. 5) indicates that a drawdown cone is now capturing water in the outcrop-subcrop area (Ryals, 1980). To accommodate the drawdown within the outcrop-subcrop area of the Sparta aquifer, the westernmost limit (nodes) of the outcrop-subcrop area was treated as constant head (pl. 2). The remainder of the outcrop-subcrop area was included as active nodes to accommodate the drawdown.

^C Initial values from Payne (1972), plate 7, were adjusted during modeling to the range shown.

d From Hosman and others (1968) and assumed values for water-table conditions.

e Initial value, 1 X 10-11 foot per second, varied during modeling to value shown.

f From Ryals (1983a); Spiers (1977); Payne (1970), plates 2 and 3; and Payne (1968), plates 3 and 4.

The eastern boundary of the model was selected as the eastern limit of the Sparta outcrop-subcrop area. The eastern boundary, like the western boundary of the Sparta, was treated as a constant-head boundary.

The boundaries of the Carrizo-Wilcox aquifer were treated the same as the Sparta aquifer. The location of constant-head nodes was slightly different to correspond to the difference in location of the outcrop-subcrop area. The justification for the way boundaries were treated for the Carrizo-Wilcox was basically the same as was mentioned for the Sparta. Different boundary conditions could be used in the Carrizo-Wilcox without materially affecting the model because the model was only calibrated on water levels for the Sparta aquifer.

The Cockfield Formation crops out over much of the study area (fig. 2), and where it does not crop out, it is overlain by alluvial deposits, or it has been removed by erosion and replaced by alluvium. The Cockfield aquifer was treated as a constant-head layer because it is in direct hydrologic connection with rivers and streams in large parts of the area. Preliminary modeling of the aquifer system showed that there was little difference between simulating the Cockfield as a constant-head layer or simulating the aquifer with the following: constant-head nodes in the outcrop area and simulating leakage into the Mississippi River alluvium or undifferentiated upper Tertiary deposits by use of the river package (McDonald, 1984). To simplify input requirements, the constant-head layer was used. The location of eastern and western boundaries of the Cockfield were selected to correspond with the eastern and western limits of its outcrop area, while the northern and southern boundaries were located the same as for the other aquifers.

Initial Potentiometric Conditions

The initial potentiometric levels used in the model for the Sparta aquifer were taken from a potentiometric map (fig. 4) of the Sparta Sand for 1886 (Reed, 1972; fig. 2), which depicts water levels prior to extensive development of the aquifer. Information on prepumping potentiometric levels for the Carrizo-Wilcox and Cockfield aquifers are not sufficient for preparing a pre-development potentiometric map; however, because the two aquifers have not had extensive development, more recent maps of potentiometric levels in these aquifers were used. potentiometric levels of the Cockfield aquifer and of the alluvial aquifer, where the Cockfield has been eroded, were obtained from the following in Louisiana, from unpublished data in the files of the U.S. Geological Survey; in Arkansas, from Terry and others (1979); and in Mississippi, from Wasson (1980). Potentiometric levels of the Carrizo-Wilcox aquifer and equivalent geohydrologic units were taken from the following sources: in Louisiana, from Ryals (1980c) and from unpublished data in the files of the U.S. Geological Survey; in Arkansas, from Terry and others (1979); and in Mississippi, from Wasson (1980).

Pumpage

The geographical and temporal distribution of ground-water pumpage from the Sparta aquifer was the only pumpage input to the detailed model. Pumpage from the Cockfield aquifer was not included because that aquifer is treated as a constant-head layer. Pumpage from the Carrizo-Wilcox aquifer was not included because of (1) the lack of long-term data on pumpage and potentiometric levels that could be used to calibrate the model and (2) the insignificant amount of pumpage in the principal area of interest in the study area.

In the detailed model, pumpage for all individual pumping centers where pumpage from the Sparta was greater than $0.15~\rm ft^3/s$ were simulated by individual wells. All other pumpage was totaled by parish and was assumed to occur at one or two nodes within each parish.

Pumpage was divided into 11 periods (table 4) based on variations in pumpage and water levels observed at West Monroe. Grid blocks used to simulate pumpage are indicated on plate 2. No pumpage is shown occurring from the Sparta aquifer prior to 1900; whereas, an estimated 248 $\rm ft^3/s$ was pumped in 1980.

Calibration of Model, 1900 Through 1964

The approach used in calibrating the model was to simulate the period 1900 through the end of 1964, making reasonable adjustments in model parameters to obtain a match between model-generated change in water levels for the simulation period and the observed change in water levels. Data used in calibrating the model were water levels in wells in the Sparta aquifer and drawdown information from maps of drawdown. Drawdown is defined as the change in potentiometric level from one time to another. For the calibration process of the detailed model, drawdown was defined as the change in potentiometric level between 1900 and 1964. Generally, maps of drawdown for 1965 were used to determine drawdown at major pumping centers outside the principal area of study in Louisiana. At most sites in Louisiana, drawdown was determined at wells. As Ouachita Parish was the principal area of interest, more wells from Ouachita Parish were used in calibrating the model than for any other area. A scattering of other wells were used in the calibration process to provide good representation of the rest of the aquifer system. Initial water levels at observation wells were estimated from maps of intial head. The potentiometric levels in wells in early 1965 were then compared to the initial head to get the drawdown in the well. Although maps of drawdown have been published for the Sparta aquifer for 1965, the period during 1965 that they represent is not known. Because a significant reduction in pumpage from the Sparta aquifer at West Monroe occurred in 1965, these published maps were not used for Louisiana. If maps of drawdown had been used to determine observed nodal values of drawdown for the calibration process, then a better calibration may have been obtained. However, the relative merit of using point values of drawdown versus nodal values of drawdown was not investigated in this study.

Table 4.--Temporal distribution of pumpage for the detailed three-dimensional model

Pumping period	Historical period simulated	Length of simulation (years)	Pumping rates from the Sparta aquifer (cubic feet per second)
1	1900-09	10	1
2	1910-19	10	2
3	1920-29	10	1.3
4	1930-39	10	40
5	1940-49	10	121
6	1950-59	10	152
7	1960-64	5	201
8	1965-69	5	21.5
9	1970-74	5	228
10	1975-79	5	239
11	1980	1	248

Initially in the calibration process, trial-and-error adjustments were made in hydrogeologic parameters (such as transmissivity and storage coefficient) in the model to match the observed potentiometric level of the Sparta aquifer at the end of 1964 and model generated potentiometric values. Because a desirable match was not made within a reasonable number of simulations, parameter estimation as described by Cooley (1977) was used to calibrate the model on the drawdown in potentiometric levels for the period 1900-1964. Parameter estimation shows the sensitivity of the model to changes in hydrogeologic parameters and, thus, aids in the calibration process. The sensitivity of the model is gaged by the change in model-generated drawdown caused by a change in hydrogeologic parameters.

Calibration of the model was based on pumping data from 1900 through the end of 1964, the first seven pumping periods shown in table 4. The ending year, 1964, was selected for calibration because of the available potentiometric data gathered for the Sparta aquifer and because of a change in pumping rate at West Monroe after 1964. During the calibration of the model, pumpage from the Sparta was not one of the parameters varied when using the parameter-estimation technique because good estimates of water use were not available for Ouachita Parish and surrounding parishes for the period 1900-1960. Prior to 1960, pumpage data were collected intermittently at some pumping centers (table 2), but for the most part, individual well data did not exist. Since 1960, records are more complete for most of the major pumping centers.

Parameter Estimation

A program written for a hand-held programmable calculator was used to determine the change in hydrogeologic parameters required to achieve a better match between observed and calculated drawdown. The program is a modified version of one documented by Torak and Whiteman (1982, appendix V). Modifications to the program documented by Torak involved eliminating

some statistical calculations and the solver subroutine. Elimination of the solver subroutine necessitated the manual solution of the governing equations. The program uses drawdown from a number of observation wells to calculate the change required in three hydrogeologic parameters to calibrate the model. Initially, 13 observation wells were used. The validity of using only 13 observation wells was later tested by adding an additional 14 observation wells. The results from all 27 observation wells are discussed below.

Transmissivity of the Sparta Sand and vertical hydraulic conductivity of the two confining layers were the first three parameters tested using the parameter-estimation technique because the model was found to be most sensitive to these parameters during the trial-and-error stage of modeling. A "base run" of the model simulating the 65 years of pumping was executed using a preliminary set of parameter values. On successive "perturbation runs" one parameter value was changed. Each perturbation run represented the effects on the aquifer system of changing one parameter.

Input to the parameter-estimation program consisted of observed drawdowns at wells and the corresponding model-generated drawdown at nodes from base and perturbation runs. The "sensistivity" or change in drawdown caused by the change in hydrogeologic parameter was computed by the program using results of base and perturbation runs. A sum of squares was then computed from the differences between observed drawdowns and those obtained from the base run. The sum of squares was used as an indicator of how close the model-generated value of drawdown was to the observed value. The lower the sum of squares the closer the match between observed and model-generated drawdown.

Values for transmissivity and vertical hydraulic conductivity were allowed to vary within plausible limits as dictated by the output of the parameter-estimation program. Parameters were changed by adjusting a multiplier value for each matrix. All elements of the matrix that defined a parameter were multiplied by this value. Additionally, trial-and-error adjustments were made within the matrices in areas where a poor match was obtained between observed and computed head.

After five sets of base and perturbation simulations, accompanied by changing of parameters as indicated by the parameter-estimation program, the sum of squares for computed and observed drawdowns were reduced by more than half of the original value. The mean difference (fig. 7) between observed and computed drawdowns decreased 5.7 ft (from 15.5 to 9.8 ft) after five simulations. Several additional simulations were made using the parameter-estimation technique. However, the change in parameters produced no appreciable change in the results as indicated by figure 7, which shows the sum of squares of drawdown and mean difference plotted versus the simulation number. After the seventh calibration simulation, the model was considered calibrated with respect to the transmissivity of the Sparta aquifer and the vertical hydraulic conductivity of the confining layers. To complete the calibration procedure, storage coefficients of the aquifers were evaluated by the parameter-estimation program. Three simulations were made, but no significant change in the sum of squares

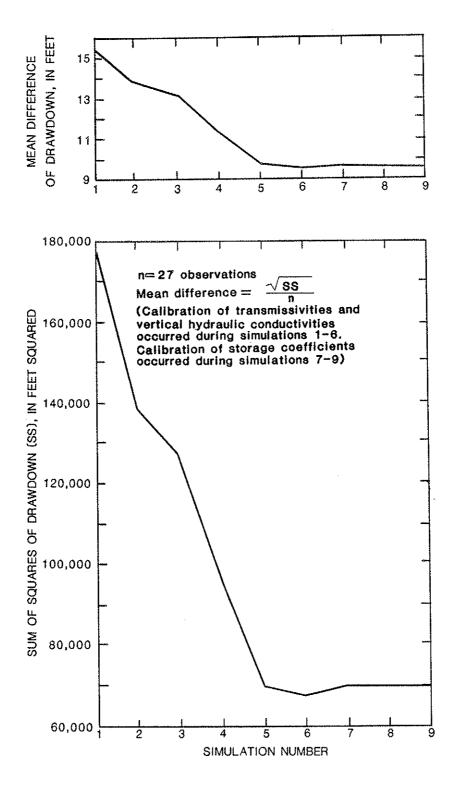


Figure 7.--Mean difference and sum of squares of drawdowns versus simulation number for calibration simulations.

occurred. After the ninth calibration run, the model was considered calibrated. Table 5 shows how the final simulation compared with the observed data. The mean difference from run nine, 9.6 ft, represents a 7 percent difference from the average observed drawdown.

During the calibration procedure, the factor that multiplies the transmissivity matrix of the Sparta aquifer was changed by 35 percent from 1.0×10^{-4} to 6.5×10^{-5} . The values of transmissivity that were input to the model were coded from a map of the transmissivity of the Sparta prepared by Payne (1968). The 35-percent variation in transmissivity from the initial estimate was considered acceptable. Additionally, the matrix values were adjusted in recharge areas in Ouachita Parish and at Magnolia, Ark. Plate 3 is included to show the areal distribution in transmissivity of the Sparta that was used in the model.

The values of vertical hydraulic conductivity for the Cook Mountain and Cane River confining layers were changed during model calibration from an initial value of 1 X 10^{-11} ft/s for both units to 1.46 X 10^{-10} ft/s and 6.08 X 10^{-11} ft/s, respectively. Whiteman (1980) reports a value of vertical hydraulic conductivity for a confining layer in the Baton Rouge, La., area of 1 X 10^{-10} ft/s. The variation in vertical hydraulic conductivity used to calibrate the model seemed reasonable, based on preliminary modeling results and the one available reported value. Changes also were made in the matrix values of the reciprocal of thickness for the Cook Mountain Formation at Magnolia, Ark. and in Ouachita Parish.

Storage-coefficient factors, which multiply matrices of values, were varied over several orders of magnitude in the last few calibration simulations with little change in computed sum of squares of drawdown. The initial factors used to multiply the matrices of storage coefficient were equivalent to storage coefficients of 0.1 for the Cockfield Formation, and 1 X 10⁻⁴ for the Sparta and Carrizo-Wilcox aquifers. Because of the lack of sensitivity, the initial values were used for the additional model calibration. Matrix values of the storage coefficient near recharge areas of the Sparta aquifer and in Ouachita Parish also were adjusted during the calibration. Generally, the lack of model sensitivity to changes in storage coefficient was (1) because of the way that sensitivity was determined and (2) because of large differences between observed and computed drawdowns in some of the wells used in the determination.

The difference between the computed and observed drawdown values in table 5 is 44 ft or more in several areas. The areas where this occurs are near pumping centers (Ruston, well L-25; West Monroe, well Ou-77; Springhill; El Dorado; and Magnolia), near the recharge area of the Sparta aquifer (well sites Sa-389 and Sa-390), and in Ouachita Parish (wells Ou-88, Ou-403, and Ou-404). Significant differences are expected when comparing point-drawdown data at or near a pumping well with the drawdown in a model-grid block because the model averages drawdown over the area; therefore, the grid-block drawdown should be less than the observed drawdown. This occurs at most of the pumping centers except for Springhill, La., which is next to the recharge area. The large difference between

Table 5.--Observed and computed drawdowns at selected nodes used to calibrate the detailed three-dimensional model

Well No. 1	Node		Observed drawdown	Computed ² drawdown	Difference	
and location	Row	Column	to 1965 (feet)	to 1965 (feet)	(feet)	(percent)
Bi-1	16	7	52	32	20	38
Bi-4	16	9	83	84	-1	1.
Cl-9, Haynesville, La	5	7	110	88	22	20
Cl-58	7	8	75	82	-7	9
Ja-49	21.	11	157	126	31.	20
L-25	12	11	162	118	44	27
Mo-5, Bastrop, La	6	26	170	190	-20	-12
Ou-24, Ouachita Parish-	8	23	140	159	-1.9	-14
Ou-77, West Monroe, La-	14	20	294	229	65	22
Ou-87, Ouachita Parish-	14	21	203	186	17	8
Ou-88, Ouachita Parish-	15	21	126	175	-49	-39
Ou-96, Ouachita Parish-	13	22	149	166	-17	-11
Ou-401A	17	23	112	136	-24	-21
Ou-403	20	22	82	115	-33	-40
Ou-404	13	26	83	134	-51	-61
Ou-406	16	21	159	162	- 3	-2
Sa-389	26	3	56	4	52	93
Sa-390	26	4	95	5	90	95
Un-26	7	20	149	153	-4	3
W-16	24	12	55	61	- 6	11
Wb-3, Springhill, La	5	4	60	1.02	-42	70
Wb-96	6	5	26	62	-36	138
El Dorado, Ark, 3	3	11	300	234	66	22
Magnolia, Ark. 3	3	7	220	41	179	81
Hodge, La. 4	20	11	160	155	5	3
Ruston, La.4	12	12	153	133	20	13
Jackson, Miss. 3	19	33	62	87	-25	-40

¹ See description of well-numbering system in the Introduction.

computed and observed drawdown at Springhill is believed to result from the difficulty of selecting appropriate transmissivity values for the varying grid-block sizes at the recharge boundary. At Magnolia, Ark., which also is near the recharge area, large differences between observed and computed drawdown occur. Reasonable adjustments in aquifer and confining-layer parameters were made, but they produced no significant improvement between observed and calculated drawdown. Because Magnolia was not an area of concern, nothing further was done to rectify differences there.

² Final simulation.

³ Based on map of drawdown (Reed, 1972).

⁴ From Sanford, 1973.

In Ouachita Parish, the difference of 50 ft or more between observed and computed drawdown is thought to be the result of two factors. gradients in Ouachita Parish are fairly steep because of pumping at West Monroe. The steep gradients could account for significant differences between observed and computed drawdown when comparing the point values of drawdown at a well to the average drawdown of the grid block in which the well is located. Another possible reason for the significant difference between observed and computed drawdown is that the Sparta aquifer is assumed to behave as a single hydrologic unit. Rogers and others (1972, p. 28) stated, "In parts of the area some of the upper sands of the Sparta, which are poorly connected with the lower sands in some places, had a piezometric surface higher than that of the deeper sands. This results from a lag in pressure adjustment to the withdrawals of water from the lower, more heavily pumped sands." Examination of the wells used for parameter estimation shows that most of them (Ou-87, Ou-88, Ou-96, Ou-401A, and Ou-403) are completed in the upper half of the Sparta Sand.

Overall, the model-computed drawdowns are considered to represent an acceptable match with observed drawdown. Given the time frame of this study, the model was considered to be adequately calibrated for the purpose of estimating the effects on water levels and salinity of the Sparta aquifer of projected increased pumpage from the Sparta near West Monroe.

Sensitivity Analysis

The sensitivity of the developed model to the various hydrologic parameters used in the model was determined through a formalized sensitivity analysis. Simulations were made in which values of a parameter were allowed to vary over a reasonable range. The mean difference of each simulation for each change in parameter was plotted versus the ratio of the varied parameter to the value of that parameter used during the final calibration simulation.

The model was found to be insensitive to changes in (1) the storage coefficient of all aquifers and (2) the transmissivity of the Carrizo-Wilcox aquifer. Large variations in these parameters would cause only a slight change in the calculated mean difference.

Figure 8A shows that the model is highly sensitive to the value of the transmissivity of the Sparta aquifer. A small change in transmissivity of the Sparta aquifer would produce a significant change in the mean difference. Figure 8C shows that the model is not as sensitive to the value of vertical conductance of the Cane River confining layer as it is to the corresponding value for the Cook Mountain confining layer, shown in figure 8B. Large changes in the vertical conductance for the Cane River confining layer produced only a negligible change in the mean difference; whereas, only a small change in the vertical conductance for the Cook Mountain confining layer produced a significant change in the mean difference.

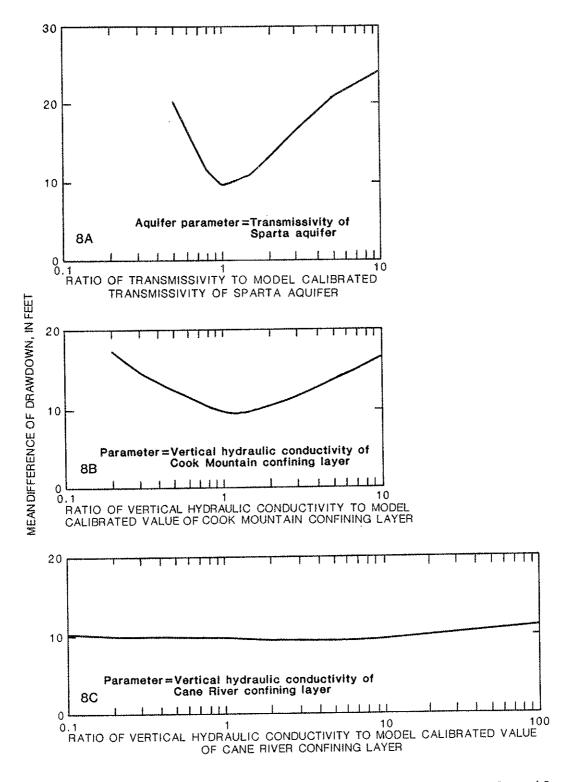


Figure 8.--Mean difference of drawdown versus ratio of range of aquifer parameter to model calibrated parameter.

To investigate the areal sensitivity of the model to the parameters used, plate 4 (maps A, B, and C) was prepared. Because the model seems to be most sensitive to transmissivity of the Sparta aquifer and vertical conductance of the Cook Mountain confining layer, the areal sensitivity of the model to these parameters was analyzed. An indicator of sensitivity is the difference between observed and computed drawdown. Plate 4A shows the areal variation between observed and computed drawdown for the Sparta aquifer for the calibration simulation through 1964. Plate 4B shows the areal variation between observed and computed drawdown for the Sparta using a value for transmissivity of the Sparta aquifer of 1.5 times the calibrated value. Plate 4C shows the areal variation between observed and computed drawdown using a value for vertical conductance of the Cook Mountain confining layer of 1.5 times the calibrated value. By comparing plate 4B and 4C to plate 4A, the sensitivity of the model to areal changes in parameters can be inferred. Vertical conductance is defined as the vertical hydraulic conductivity of the confining layer divided by the thickness of the layer.

Plate 4A shows a fairly random distribution of positive and negative drawdown differences between the observed and computed drawdown. The one exception to the random distribution is in Ouachita Parish, where mainly negative differences occur. During the parameter-estimation simulations, matrix values of vertical conductance for the Cook Mountain confining layer and the matrix values for transmissivity in the Sparta aquifer in Ouachita Parish were increased as much as 50 percent. The modification of the nodal values of transmissivity and vertical conductance resulted in some decrease in the difference between the observed and computed head. Areal variations beyond 50 percent in aquifer parameters were not considered reasonable.

From plate 4B and 4C, it appears that the model was areally sensitive to increases in the vertical conductance of the Cook Mountain confining layer and transmissivity of the Sparta aquifer. Comparison of plate 4B and 4C with 4A shows that in Ouachita Parish increases in the two parameters would result in a decrease in the difference between observed and computed drawdown. Elsewhere, either no significant change occurs, or the difference between observed and computed drawdown increases. sensitivity analysis indicates that a decrease in the difference in observed and computed drawdown could have been obtained with further areal adjustment of parameters in Ouachita Parish. However, additional areal adjustment was not made in the two parameters under consideration because the areal distribution of parameters used for the calibration through 1964 was considered reasonable. More importantly, further areal adjustments were not made in the parameters because the change in drawdown would not have significantly improved the accuracy of the model as measured by the mean difference. Computed drawdown from the calibration simulation through 1964 was already within the error range for the observed drawdown.

The sensitivity analysis indicated the adequacy of the aquifer parameters used in the model. The model could be refined if better estimates of the magnitude and areal distribution of transmissivity of the Sparta aquifer and leakance of the Cook Mountain confining layer were available.

Steady-State Model Simulation

The calibrated model was used with the initial potentiometric conditions, discussed previously, to generate a steady-state surface of initial potentiometric levels of the Sparta aquifer. The computed initial potentiometric levels shown on plate 5 are roughly comparable to the observed potentiometric levels shown in figure 4. Near West Monroe, the computed potentiometric levels are higher by 20 to 30 ft than the observed initial potentiometric levels. Generally, the computed steady-state potentiometric surface ranges from 0 to 40 ft higher than the observed potentiometric surface. Because of the lack of control for the observed potentiometric surface, this degree of difference was deemed acceptable. Drawdown computed during additional model simulations were superimposed on this steady-state potentiometric surface to generate potentiometric surface maps for other time periods.

Acceptance Testing of the Model, 1965 through 1979

To further test the developed model to determine its accuracy for analyzing the results of increased pumpage near West Monroe, the model was further checked against the decline in water levels in the Sparta aquifer from 1965-79. This acceptance testing involved determining if the model could match recorded changes in water level using pumpage from a period of time different than that used to initially calibrate the model. Pumpage was divided into four pumping periods, the last four pumping periods shown in table 4. The periods were divided so that drawdown data from the end of 1979 could be compared with observed data from early 1980. Simulations used to test the model were a continuation of the last calibration simulation. Drawdown at the end of 1964 was input to the simulation as starting head. Except for the change in pumping rates, the two simulations were identical but were independent of each other.

Table 6 shows the computed versus observed drawdown for the acceptance testing simulation for 21 observation wells. The sum of squares and mean difference of the simulation were 6,148 ft² and 3.7 ft, respectively. The mean difference is significantly less than that calculated during the earlier calibration. Table 6 shows that more than 50 percent (3,844 ft2) of the total sum of the squares of the difference between the observed and computed drawdown is a result of data for well Ja-49, which is near the pumping center at Hodge, La. Records of pumpage near well Ja-49 show that pumpage increased significantly after 1960, from 16 ft3/s to 20-22 ft³/s and then remained fairly constant through 1980. On the basis of the pumping rates, an increase in drawdown would be expected from 1965-80; yet, decreasing drawdown (rising water level) occurred in well Ja-49. The apparent anomaly is believed to be the result of a shift in pumpage among wells within the grid block. Eliminating the values for well Ja-49 results in a mean difference of 2.4 ft for the remaining wells in table 6.

Table 6.--Observed and computed drawdown at selected nodes (1965-79) used during second calibration of the detailed three-dimensional model

[Sum of squares = 6,148 feet and mean difference = 3.7 feet]

Well No.	No	xde	Observed drawdown	Computed drawdown	Dif:	ference
werr no.	Row	Column	1965-80 (feet)	1965-80 (feet)	(feet)	(percent)
Mo-5	6	26	0	24	-24	
Ou-24	· 8	23	13	19	- 6	46
Ou-77	14	20	$\mathtt{a}_{\mathtt{-l}}$	10	-11	1,100
Ou-88	15	21	10	20	-10	100
Ou-96	13	22	30	22	8	27
Wb-27	5 5	4	a_33	-26	~7	21.
CL-9	5	7	1.6	15	1.	6
Bi-l	16	7	18	16	2	11
Bi-4	16	9	41	30	11	28
Cl58	7	8	12	26	-14	. 117
Ja-49	21	11	a_21	41	-62	295
L-25	12	11	19	41	-22	115
Ou-401A	17	23	24	21	3	13
Ou-403	20	22	11	21	-10	91
Ou-404	13	26	15	21	~6	40
Ou-406	16	21	14	21	- 7	5 0
Sa-389	26	3	a_3	1 2 23	-4	133
Sa-390	26	4	0	2	~2	
Un-26	7	20	14	23	-9	64
Wb-96	6	5	a-4	~ 5	1	25
W-16	24	12	7	23	-16	228

a Water-level rise.

Comparison of observed versus computed drawdown in wells in Ouachita Parish, listed in table 6, shows a sum of squares of 515 ft² and a mean difference of 2.8 ft for the eight wells. The mean difference represents a 19-percent difference from the observed average drawdown of 14.5 ft in the eight observation wells. The largest differences between computed and observed drawdown in Ouachita Parish are in wells Ou-77, Ou-88, and Ou-403. As stated earlier, Ou-88 and Ou-403 are wells screened in the upper part of the Sparta Sand; whereas, most of the pumpage is from the lower part of the Sparta. The large difference between observed and computed drawdown in these wells may result from a time lag in response to pumping if upper sands of the Sparta are poorly connected to the lower sands. The poor match between computed and observed drawdown in well Ou-77 is probably because the comparison is between the average drawdown in the grid block and the point drawdown at the actual well.

During the model simulation period, pumping was reduced at several of the major pumping centers. At Springhill, pumpage was reduced from 16.70 ft 3 /s in 1965 to 0.90 ft 3 /s in 1980. In the Monroe-West Monroe area, pumpage decreased from 22.97 ft 3 /s in 1965 to 15.80 ft 3 /s in 1970. Then from 1970 to 1980, pumpage at Monroe-West Monroe increased to 22.95 ft 3 /s. The observed water-level rise for the period 1965-80 was 33 ft at Springhill and 1 ft at West Monroe. The model computed a 26-foot rise at Springhill and 10-foot decline at West Monroe.

Plate 6 shows the computed potentiometric level of the Sparta aquifer at the beginning of 1980. Drawdown data through the end of 1979 were combined with the computed steady-state water level to prepare plate 6. Comparison of plate 6 with figure 5 shows how well the computed potentiometric levels agree with the observed. The computed potentiometric level does not show a cone of depression around Magnolia, Ark. As noted previously, values of aquifer and confining-layer parameters around Magnolia were varied in the modeling effort within plausible limits; yet, a drawdown cone similar to the observed cone could not be produced. The shape of the drawdown cones around the other pumping centers indicated by the computed potentiometric-contour map are similar to those observed. The calibrated model, when tested for an additional time period, reasonably reproduced drawdowns for that period and is deemed acceptable for evaluating various pumping alternatives proposed by the Corps of Engineers.

Figure 9 shows the computed versus observed drawdown at wells Ou-77, Ou-87, and Ou-96. The hydrographs show how well the model reproduced the trends in water levels in selected wells in the principal area of interest in Ouachita Parish. Well Ou-77 is located in the center of the major pumping cone at West Monroe. Close agreement between observed and computed drawdown at well Ou-77 was not expected because the comparison is between the point value of drawdown in a well in a pumping field versus the average value of drawdown over an area of the node, which is 4 mi2. Figure 9 shows that for well Ou-77 for early time (1940-60), when only scanty data on pumpage were available, the difference between the computed and observed drawdown was greater than in later time (1960-80). Beginning in 1960 when records of pumpage improved, the computed and observed drawdown trends are very similar although there is a water-level difference of about 40 ft. The observed versus computed drawdown at well Ou-87 also shows a poor match between observed and computed drawdown for the early data and a better match from 1960 to 1980. It appears that the results of the ground-water model would be improved by better definition of pumpage during earlier time periods (1900-1960). Well Ou-96, which is farther out on the cone of depression surrounding West Monroe than either well Ou-77 or Ou-87, has shown a steady drawdown of about 2 to 3 ft/yr during the period of record. The model-computed drawdown for well Ou-96 closely approximates the observed drawdown.

The drawdown computed from the last calibration simulation was sufficiently close to the observed drawdown that no additional adjustments were necessary. The total mean difference from the combined calibration simulations is 9.6 ft, which is less than 10 percent of the average drawdown for the 1900-1980 period. The calibration and acceptance testing simulations indicate the model satisfactorily reproduces drawdown within

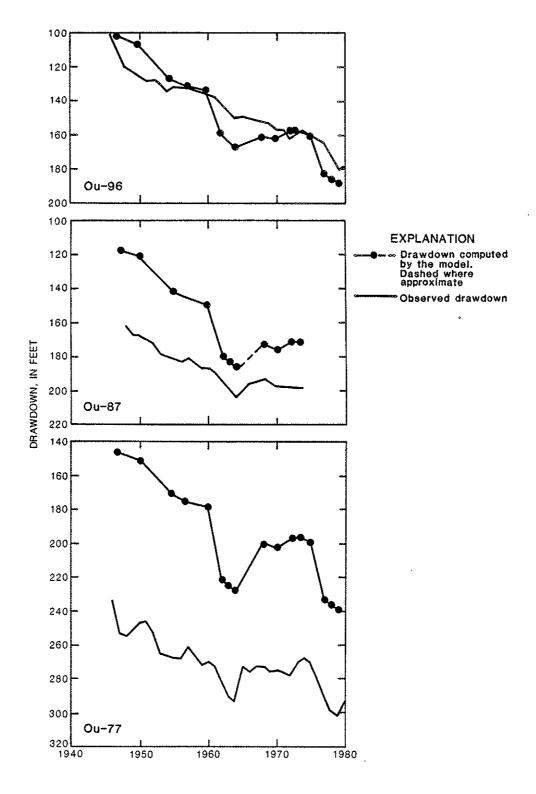


Figure 9.--Observed drawdown versus computed drawdown for selected observation wells near West Monroe, Louisiana.

the Sparta and is, therefore, suitable to analyze the drawdown effects of projected increased pumpage. The water budget was examined to further evaluate the validity of the model and the impact of drawdown within the Sparta outcrop areas capturing ground water discharge to streams.

Evaluation of Components of the Flow System

In 1980, 247.8 ft³/s was pumped from the Sparta aquifer (table 7). Of this amount, 40 percent was derived from the outcrop-subcrop area of the Sparta; 44 percent was derived from leakage into the Sparta from the Cockfield aquifer through the overlying Cook Mountain confining layer; 5 percent was derived from leakage from the Carrizo-Wilcox aquifer through the underlying Cane River confining layer; and 11 percent was derived from release of water from storage in the Sparta aquifer. The 40 percent of pumped water that was derived from the Sparta outcrop-subcrop area in 1980 represents a "captured-recharge" rate of 0.3 in/yr. The water is captured recharge because this represents water that formerly was discharged to streams in the recharge area. Although this captured-recharge rate cannot be verified, it is not unreasonable in terms of 50 to 60 in/yr of rainfall. Because the model was not calibrated on water levels in either the Carrizo-Wilcox aquifer or Cockfield aquifer, the percentages of leakage from the individual layers are somewhat suspect.

Cumulative pumpage from the Sparta aquifer for the period 1900-1980 was 2.51 x 10¹¹ ft³ (table 7). Of this amount, 38 percent was derived from the recharge area of the Sparta; 40 percent was derived from leakage into the aquifer through the Cook Mountain confining bed; 5 percent of the water was derived from leakage through the Cane River confining bed; and 17 percent was derived from release of water from storage in the Sparta. The total leakage (45 percent) is considered a valid number. Reed (1972), using an analog model of the entire Sparta Sand, found that in 1965, with a pumpage of 539 ft³/s, 60 percent of the water was derived from leakage, 20 percent from captured recharge, and 20 percent was released from storage in the Sparta. The value from the current study compares well with Reed's analog-model results for the entire Mississippi embayment. Based on the simulations and on the evaluation of the water budget, the model was considered suitable for analyzing the effects of increased pumpage.

SIMULATED AQUIFER RESPONSE TO ALTERNATIVE PUMPING PLANS

Projected Pumpage

The U.S. Army Corps of Engineers initially requested that the effects of obtaining approximately 20 ft³/s from three well fields west of West Monroe be analyzed. The model described in the previous section was designed to analyze the effects of this pumpage. After review of the results of the initially proposed pumping plans, the Corps of Engineers requested that the effects of eight alternative pumping plans on water levels and water quality of the Sparta aquifer be analyzed. Table 8 shows the additional ground-water requirements in the West Monroe area projected

Table 7.--Volumetric budget for the Sparta aquifer

Inflow	1980 (cubic feet per second)	Cumulative 1900-1980 (cubic feet)
Storage	26.9 98.9 109.6 13.6	4.38 X 10 ¹⁰ 9.42 X 10 ¹⁰ 1.00 X 10 ¹¹ 1.33 X 10 ¹⁰
Total	249.1	2.51 X 10 ¹¹
Outflow	1980 (cubic feet per second)	Cumulative 1900-1980 (cubic feet)
Storage Leakage through the Cane River Formation Pumpage from wells	0.8 .4 247.8	9.96 x 10 ⁷ 3.61 x 10 ⁸ 2.51 x 10 ¹¹
Total====================================	249.0	2.51 X 10 ¹¹
Percentage of error	.0	.0

by the Corps of Engineers. The table shows six groups of wells (A through F), the location of wells by model-node location (row, column) within each group, and the projected pumpage at each node with time. Plate 7 shows the location in Ouachita Parish of the wells listed in table 8. Groups A, B, and C represent existing pumpage (1980), and the pumpage from these wells was the same as was used in the simulation for 1980. Groups D and F represent new pumpage in 1980 and, therefore, were not applied except during simulation of the alternative plans. For the eight alternative model simulations, projected pumpage was based on eight different combinations of the well groups shown in table 8 and described below.

Alternative A.--Pumping would consist of groups A, B, and C from table 8. Groups A, B, and C show an increase in pumpage from 12.03 ft 3 /s in 1980 to 22.61 ft 3 /s in 2040, an 88-percent increase.

Alternative B.--Pumping would consist of groups A, B, C, and D in table 8. Group D would involve a large increase in pumpage from no pumpage in 1980 to 23.47 $\rm ft^3/s$ in 2040. The total increase in pumpage from the 1980 value is 34.05 $\rm ft^3/s$.

Alternative C.--Pumping would consist of groups A, B, C, and E in table 8. Group E would involve an increase in pumpage of 8.54 ft³/s in 2040. The total increase in pumpage from the 1980 value is 19.12 ft³/s.

Table 8.--Temporal distribution of projected pumpage for simulating alternative plans

1 '		Rate	1 .	of pumping (cubic feet per	cubic f	eet per	second)	per	eriod i	period interval	(years)		
1980	` '	1981- 85	1986 - 90	1991- 95	1996- 2000	2001- 05	2006- 10	2011- 15	2016- 20	2021- 25	2026 - 30	2031- 35	2036- 40
				•	•	Group A							
00.00		0.00	00.00	0.14	0.24	0.34	0.44	0,54	0.64	0.74	0.84	0.94	0.94
80.		90.	80.	.14	. 24	.34	. 44	.54	.64	.74	.84	.94	.94
000		00.	00.	00.	• 05	TO.	00.	00.	•05	.17	.39	19	.94
00.		.34	• 44	. 54	. 64	.74	.84	.91	.94	94	.94	- 94	· 94
1.08		1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08
00.		00.	00.	00.	• 04	.02	TO.	10.	.05	.17	.39	.62	.94
00.		.34	44	. 54	.64	.74	84	<u>.</u>	.94	. 94	.94	- 94	-94
1.08		1.08	1.08	1.08	1.08	1.08	1.08	1,08	1,08	1,08	1.08	1.08	1.08
		3,26	3,26	3,26	3.26	3.26	3.26	3,26	3.26	3.25	3,26	3,26	3,26
		00.	.24	.34	• 44	. 54	• 64	.74	. 84	. 95	. 95	.95	.95
		00.	.24	.34	. 44	. 54	. 64	.74	.85	94	94	• 94	.94
			٠			Group B							
0.93		0.97	1.01	1.06	1.09	1.13	1.16	1.19	1,22	1.30	1,38	1.46	1.54
. 93		.97	1.02	1.06	1.10	1.13	1.16	1.19	1.22	1,30	1,38	1.46	1,54
• 94		. 98	1.02	1.06	1.10	1.13	1.16	1.20	1.23	1,31	1.38	1.47	1,54

^a An additional 17.86 cubic feet per second of pumpage was included at model row 14, column 20, to account for water use by the papermill at West Monroe.

Table 8. -- Temporal distribution of projected pumpage for simulating alternative plans -- Continued

Model.	Mode1		Rato	e of pu) buidum	cubic f	e of pumping (cubic feet per		per p	eriod i	second) per period interval (years)	(years	(
row	column	1980	1981- 85	1986- 90	1991- 95	1996- 2000	2001- 05	2006-	2011- 15	2016- 20	2021- 25	2026- 30	2031- 35	2036-
							Group C							
11	22	96.0	1.00	1.01	1.02	1.03	1.04	1.05	1.06	1.07	1.05	1.02	0, 99	0.96
11	24	90.	00.	.11	.12	.13	.14	.15	•16	.17	.18	919	.20	ਨ ਂ
11	22	8.	90.	0.	.04	.08	.08	.08	60.	.10	Į.	,12	,13	.14
12	23	96•	1.00		1.02	1.03	1.04	1,05	1,06	1.07	1,04	1,01	. 86*	96.
15	22	96•	1.00		1.02	1.03	1.04	1,05	1.06	1,07	1,04	1.01	- 38	.95
15	23	90.	00.	00.	.03	90.	.07	80.	60.	.10	T.	,12	.13	.14
17	23	00.	00.		• 03	90.	.07	.08	60.	.10	Ħ.	.12	.13	.14
							Group D							
T	18	14.93	15.60	16.27	16.94	17.62	18.23	18,83	19,43	20.04	20.89	21.75	22,61	23.47
						-	Group E							
Ħ	18	0.00	0.67	1,34	2.01	2.69	3,30	3.90	4.50	5,11	5,96	6,82	7,68	8,54
						-	Group F	:						
11	15	14.93	15.60	16.27	16.94	17.62	18.23	18,83	19,43	20,04	20,89	21.75	22,61	23,47

Alternative D.--Pumping would consist of groups A, B, C, and F in table 8. Group F would involve an increase in pumpage of $23.47 \text{ ft}^3/\text{s}$. The total increase in pumpage from the 1980 value with this alternative plan is $34.05 \text{ ft}^3/\text{s}$.

Alternatives Al, Bl, Cl, and Dl.--Pumping would be the same as for alternatives A-D, except that the pumpage would increase at all other pumping centers from the Sparta aquifer, assuming that the growth rate for those centers was the same as for alternative A.

Groups A, B, and C represent existing pumping centers in Ouachita Parish in 1980. Projections by the Corps of Engineers indicate that from 1980 to 2040 pumpage for alternative A (groups A, B, and C) will increase 88 percent. Therefore, for alternatives Al through Dl to include growth outside Ouachita Parish, background pumpage was increased by 88 percent from 1980-2040. The pumpage from the Sparta aquifer outside Ouachita Parish in 1980 was 218 ft³/s. This background pumpage in 2040 for alternatives Al through Dl will increase to about 410 ft³/s.

Analysis of Impact on Potentiometric Levels

Simulations of alternative pumping plans were made using the eight pumping schemes described previously. The drawdown from 1900 to 1980 was input to these simulations. The cutput of the simulations was drawdown from 1980 to 2040. This output was superimposed on the prepumping water level of the Sparta aquifer (obtained from the steady-state analysis) to obtain the potentiometric level in 2040.

Alternatives B and Bl produced the most severe impact on the potentiometric levels of the Sparta aquifer. The results of the model simulations for these alternatives in the area of interest, Ouachita Parish, are shown on plates 8 and 9. These alternatives caused a severe impact because of the magnitude of the pumpage and the location of a new major pumping center (group D, table 8) adjacent to the existing major center at West Monroe. The most severe impact of these two alternatives was caused by Bl because a projected increase in regional pumpage also was included.

For alternative B, the drawdown cone at West Monroe is projected to decline to 236 ft below sea level at the end of 2040, from the model-generated level of 96 ft below sea level at the beginning of 1980. Additionally, a new drawdown cone is projected to develop around pumping center group D (table 8, pl. 7), northwest of West Monroe. The water level at this new drawdown cone would be 250 ft below sea level at the end of 2040, a decline of 211 ft from early 1980. For alternative Bl, the water levels at West Monroe would deline to 327 ft below sea level at the end of 2040. The water level at the group D well field would decline to 350 ft below sea level by 2040, which represents a 311-foot decline from 1980.

The top of the Sparta aquifer in Ouachita Parish ranges from less than 100 ft below sea level to more than 200 ft below sea level. Alternative B and Bl are projected to cause the potentiometric level of the Sparta aquifer in Ouachita Parish to drop below the top of the aquifer. For

alternative B, the potentiometric level in Ouachita Parish will drop below the top of the aquifer in the area within the -150 ft contour line (pl. 8). For alternative Bl, large water-level declines would occur, and the potentiometric level would drop below the top of the aquifer in large areas of the model, including most of Ouachita Parish (pl. 8). With plan Bl, dewatering in the upper part of the aquifer probably will occur. Dewatering occurs when the potentiometric level of an aquifer drops below the top of the aquifer and the aquifer changes from a confined system to a water table, or unconfined, aquifer. Problems associated with large water-level declines are land subsidence and possible well failure as a result of the subsidence and changes in the aquifer properties within the areas where dewatering occurs, such as lowered aquifer transmissivity, higher storage coefficient, and lower well yield.

Plates 8 and 9 also show the projected potentiometric surface of the Sparta aquifer for the year 2040 in Ouachita Parish for the remaining six alternative pumping plans. All of the alternatives are projected to result in potentiometric levels in the Sparta aquifer dropping below the top of the aquifer in Ouachita Parish. The conditions projected by plans Al through Dl will have a greater impact than plans A through D. For plans Al through Dl, the potentiometric level will drop below the top of the aquifer in larger areas. For plans A-D, plans B and D are projected to cause the most severe areal drawdown and plans A and C would result in the least. Alternative A minimizes the areal extent of drawdown below the top of the aquifer.

The ground-water flow model used in this study was not set up to treat the local change from a confined to unconfined aquifer, as occurred with the eight alternative pumping plans. To determine when the potentiometric level drops below the top of the aquifer, elevations of the base of the Cook Mountain confining layer (Ryals, 1984) were compared with potentiometric levels. The model did not treat the change in storage coefficient that occurs when the level declines below the top of the aquifer. This change would be from the model value of about 1 X 10-4 to a value between 0.1 to 0.3. Drawdown associated with the change in storage coefficient likely would be an order of magnitude smaller because water would begin to be released from storage. The model results, thus, represent a "worstcase" condition. At West Monroe, most of the pumpage from the Sparta is from the lower part of the aquifer. As pointed out during the discussion of calibration, the uppermost sands typically are poorly connected to the lower sands. This resulted in model simulated drawdown being greater than the observed drawdown. Because the model was not designed to treat the poor connection between upper and lower sands that occurs in some areas, the model-projected drawdown in these areas also would represent a worstcase condition. Despite these limitations, the results discussed above provide a means for evaluating the relative effects of each of the alternative pumping plans.

Analysis of Movement of Saline Water

The pumping alternatives projected by the Corps of Engineers will have an adverse impact on the rate of movement of saline water within the Sparta aquifer toward West Monroe. Presently (1983), saline water occurs in most of the Sparta about 6 mi southeast of West Monroe. The rate of movement of the saline water toward West Monroe was estimated to be about 150 ft/yr during a previous investigation (Rogers and others, 1972, p. 31). However, based on information from this model, different projected rates of movement have been made for 1980 and for each of the pumping alternatives. Rates of water movement were estimated using Darcy's law, hydraulic parameters used in the model, projected potentiometric levels along a flow path across the saltwater front (model nodes row 14, column 20 and row 17, column 23), and an assumed effective porosity of 0.10 and 0.30. The form of Darcy's Law used in these calculations is

$$V = \frac{(3.1356 \times 10^7) T}{N_e b} \frac{dh}{dl}$$

where

V = average velocity of ground-water movement in feet per year

N_e = effective porosity of the aquifer as a decimal fraction (dimensionless)

T = the transmissivity of the aquifer along the flow path in square feet per second.

b = thickness of the aquifer (feet)

 $3.1356 \times 10^7 = \text{conversion factor from feet per second to feet per year.}$

Using an effective porosity of 0.1 (lower limit of probable range) results in a faster rate of water movement; whereas, using an effective porosity of 0.3 (upper limit of probable range) results in a slower rate. In the following discussion of the effects of pumpage on the rate of water movement, the rate using an effective porosity of 0.1 is given first, followed in parentheses by the rate using an effective porosity of 0.3.

The rate of saline-water movement toward West Monroe for 1980 is estimated to have been 96(32) ft/yr. For 2040, the maximum rates of movement toward West Monroe were obtained for pumping alternatives B and Bl. For alternative B, the rate in 2040 is projected to be 156(52) ft/yr; for alternative Bl, 167(56) ft/yr. Alternative A produced the slowest rate of movement (129(43) ft/yr). Based on these data, the rate of movement of saline water toward West Monroe will increase between 34 and 74 percent from the 1980 rate. However, no adverse effects on the water quality in West Monroe from the saltwater body to the southeast is anticipated for at least 200 years. Wells closer to the saline water would be affected sooner. For more accurate predictions of the movement of saline water toward West Monroe, a solute-transport model, which is beyond the scope of this investigation, would be needed.

SUMMARY AND CONCLUSIONS

A conceptual model of the Sparta aquifer was developed using preliminary two- and three-dimensional finite-difference ground-water models. The preliminary modeling indicated that the Sparta aquifer does not behave as an isolated hydrogeologic unit when pumped because significant amounts of water are derived from underlying and overlying aquifers as leakage through intervening confining layers.

A detailed three-dimensional finite-difference ground-water model was developed to simulate flow in the system that included the Sparta and underlying and overlying confining layers and aquifers. The area modeled extended from the eastern outcrop area of the Sparta aquifer in Mississippi, across northern Louisiana to its western outcrop. The model was calibrated for the period 1900-1964, using parameter-estimation techniques. developed model was found to be most sensitive to the transmissivity of the Sparta aquifer and the vertical hydraulic conductivity of the Cook Mountain confining layer. The model was further tested for water-level response to pumpage for the period 1965-79. A water budget of the Sparta aquifer, based on results from the model for the period 1900-1980, indicates that about 45 percent of the water pumped from the aquifer is derived from leakage through overlying and underlying confining layers. Of the remaining 55 percent of water pumped from the aquifer, 38 percent originates from the outcrop-subcrop areas and 17 percent originates from release of water from storage in the Sparta aquifer. The major source of leakage to the Sparta aquifer is from the Cockfield aquifer, which is the source of 40 percent of the water pumped from the aquifer.

The U.S. Army Corps of Engineers has projected increased pumpage from the Sparta to meet water-supply requirements for West Monroe. The model was used to determine the effects of these increases on water levels and water quality. Eight alternative pumping plans were evaluated using the model. The additional pumpage for the period 1980-2040 projected for the eight plans ranged from 10.58 ft³/s for plan A to 226 ft³/s for plan Bl. In all instances, potentiometric levels of the Sparta aquifer were projected to decline below the top of the aquifer within parts of Ouachita Parish. The maximum decline in water level, for the period 1980-2040 associated with the eight pumping plans, ranged from 51 ft (plan A) to 296 ft (plan Bl). These projected drawdowns represent the worst-case conditions because the model did not treat the change from confined to water-table conditions or the local poor connection within sands of the Sparta aquifer.

Water levels projected by the model were used to calculate the rate of movement toward West Monroe of saline water in an area 6 mi southeast of West Monroe. To bracket the potential rate, effective porosities of 0.1 and 0.3 were assumed in the calculations. In the following discussion the rate determined by using a value of 0.1 is given first, followed in parentheses by the rate using 0.3. The 1980 rate of movement was 96(32) ft/yr. The rate of water movement for 2040 for the eight plans ranged from 129(43) ft/yr for plan A to 167(56) ft/yr for plan Bl. The projected pumpage is not expected to cause adverse water-salinity problems at West Monroe for at least 200 years. Of the eight plans evaluated, plan A is

preferable because it produced the least drawdown, minimized the areal extent of drawdown, and produced the slowest rate of movement of saline water towards West Monroe.

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APPENDIX A

Listing for Sparta Sand, Calibration Run 9

Data are input to the McDonald (1983) modular model through the various modules in the program. Input for each module used during the final calibration simulation are listed on the following pages.

- Al. Basic Package Input Listing
- A2. Block-Centered Flow Package Input Listing
- A3. Well Package Input Listing
- A4. Strongly Impicit Procedure Package Input Listing
- A5. Output Control Listing.

Al. BASIC PACKAGE INPUT LISTING

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2166.000	2398.000	1934.000	1392.000	1392.000	1779.000	2050.000	1470.000
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3.57E-3 3.45E-3 3.17E-3 3.08E-3 3.12E-3 3.17E-3 3.45E-3 3.08E-3 3.12E-3 3.33E-3
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АЗ.	WELL PACKAGE	INPUT LISTING	CONTINUED
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A3.	WELL PACKAGE	INPUT LISTING-	CONTINUED
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\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	111111122333555566666666666777783899011	12 23 45 26 20 27 11 11 45 75 67 80 11 11 46 81 12 42 11 12 13 14 16 16 17 16 17 16 17 17 17 17 17 17 17 17 17 17 17 17 17	-3.8543 -3.443 -20.755 -0.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500 -1.500

### A3. WELL PACKAGE INPUT LISTING--CONTINUED

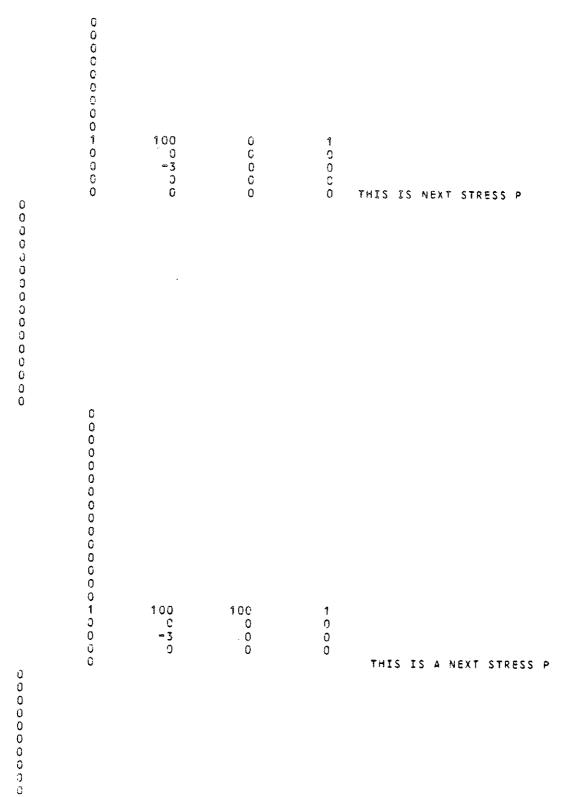
2	11	19	~0.31
2	12	7	<b>~</b> 0.22
2	12	9	-0.56
2	12	12	-3.10
2	12	23	-0.15
2	13	11	-0.12
2	13	19	~1.85
٤	14	19	-0.46
2	14	20	-20.61
2	50	11	-16.45
2	21	11	-0.62
2	24	9	-1.31
2	24	11	~0.90
S	24	12	-1.08
2	1	3 2	-6.00
2	3	32	-0.20
2	5	32	~13.9j
\$	3	33	-1.15
2	18	33	-1.00
	19	33	-16.00
2 . 2	20	33	-1.00
2	19	34	-0.20

# A4. STRONGLY IMPLICIT PROCEDURE PACKAGE INPUT LISTING

		AS.	OUTPOT CONTR	OU TITOTT	TAG
0000000000000000	<b>~7</b> 0	-7 0	4 C O	0	
0	000000000000000000000000000000000000000	0	0 0 0	0	
000000000000000	0 1 0 0 0	0 100 0 -3 0	00000	0 1 0 0 0	THIS'IS NEXT STRESS P
Ö	0 0 0 0 0				

A5. OUTPUT CONTROL LISTING

#### A5. OUTPUT CONTROL LISTING-CONTINUED



#### A5. OUTPUT CONTROL LISTING--CONTINUED

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000000000000000000000000000000000000000	000000000000010000
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103 C 0	100 0 0
1 0 0	1 0 0 0
	THIS IS NEXT STRESS P

# ე 0 0 C ŋ THIS IS NEXT STRESS P 0000000000000000 1 1 1 100 0 -3 0 100 0 0 0 1 0 0 0 THIS IS NEXT STRESS P Ü 0000000 00000000 000000

A5. OUTPUT CONTROL LISTING--CONTINUED

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0	1	

A5. OUTPUT CONTROL LISTING--CONTINUED

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0	1		
0	1		
0	1		
1	100	100	î
3	3	11	0
3	<b>~</b> 3	11	0
3	3	11	0

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•				
•				
	•			
Sec. Object				
<del></del>	·			
•				
-				
•				
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#### APPENDIX B

# Listing for Sparta Sand, Steady-State Simulation

Data input for the steady-state model simulations were the same as for the last calibration simulation except for:

- Bl. Basic Package Input Listing
- B2. Output Control Listing (No well package is used.)

#### B1. BASIC PACKAGE INPUT LISTING

ş: •

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SPARTA SAND, STEADY STATE RUN S1=.1
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10-10 BY DOUG TRUDEAU
 3
   26
        1
          1
    29
11
     30
   1
      (3512)
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  0
  0-1-1-1-1-1-1-1-1-1-1-1-1-1-1
            0
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   0
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        -3
  -1 1 1 1 1 1
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        111111
      1 1 1 1
 1 1
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         1 1 1
 0-1
  1
  1
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   1
    1111111
  111111111111
 0-1 1 1
   0-1-1-1-1-1 1 1
   111111111111111111111111111111
-3
     (3512)
```

#### B1. BASIC PACKAGE INPUT LISTING--CONTINUED

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											220	550	175	150	175	500	250	250	225
		125	100	90	90	90 325	110 275	110 250	90 225	250	105 150	15C	90	7.5	75	80	95	80	90
80	75	70	70	70	85	90 290	101	100	90 200	95 190	93 175	160	200	210	200	150	100	85	80
75	74	74	74	73	74	75		100	100	90	86								
70	69	69	70	71	72	73	75	180 79	8.5	170 90		120 220							90
70	60	59	60	69	70	72	74	180 72	155 80	145 85	105 85	80 220	100	129	130	123	112	100	85
65	59	58	59	63	68	72	72	220 70	190 76	175 84		110 220	79	77	80	82	25	3 5	80
								230	230	200	180	160	120	100	3 5	79	75	70	69
60	54	54	58	60	65	71	71	69 250	70 260	83 220		220 190	130	119	100	91	30	75	66
56	50	51	5 5	60	64	69	71		280			210 190	137	120	1 <b>1</b> 0	100	95	81	65
54	49	51	5 5	60	64	68	70	70	69	82	90	200							
5 2	48	51	5 5	59	62	65	68	70	68	32	90	190 130	330				3 C	30	63
50	48	52	5 5	56	60	62	65		300 67	250 82		190 180		120	110	9.9	90	73	60
49	47	52	53	5.5	58	60	64		300 66		224	19G 130	140	120	110	94	35	75	6 C
											225	190	150	120	110	95	54	71	58
47	47	52	53	5 5	56	60	63		65	79	230	180 200	153	120	110	99	<b>54</b>	70	5 5
46	46	51	5 2	53	55	60	63	<del>5</del> 5	64	78		180 200		125	110	100	80	55	55
46	46	51	51	52	54	60	63	63	63	77		180							

#### B1. BASIC PACKAGE INPUT LISTING-CONTINUED

45	45	49	49	50	53	59	62	61	63	76		190 180		130	108	100	80	65	5 5
44	44	47	47	50	53	58	61	60	62	75		190		143	115	100	30	65	53
43	43	44	44	47	53	5.5	59	61	61	73	200		155 365	145	118	100	80	65	50
39	39	42	40	43	50	53	55	60	60	71	210		155	148	125	100	. 80	65	50
39	38	38	37	39	44	50	53	59	60	70	205	190		150	130	105	80	65	50
45	39	38	38	38	39	40	48	55	60	. •		180		150	120	100	80	65	55
60	5.5	45	39	38	38	39	42	49	55			160		110 290	90	80	75	70	65
110	100	90	80	70	60	50	40	40	42		100		100	100	100	110	110	110	110
	110	90	60	50	40	35	33	90 33	80 33	90 35	140	140		100	100	120	130	130	130
40	40	39	220 38	219	-	218		130	150	110	80	70	60	58 140	5 5	50	48	45	40
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150	150	150	150	150		146 210		143	138	137	138	500							
140	140		140 237		137		130		125		137	200				128	-		
124 245	123	122		119	117		108	103	100	115	135	220				129			
	117	115		111	110	106		99	200 86		150	550	310						
118		114	112		107	104	99	93	97	105	155	230	315			130			
118	116		112	109	105	103	99	98			160	237	315			129			
118	115	112	110		105	100	99	98	96 205	105	165	240	315			131			
118	116	113	110	107	104	100	98	97	96	105	173 165 173	240	315			130			
118	116	113	110	103	106	99	98 214	97	95	105	165	240	310	350		130			
113	116	113	110	107	105	99	98 223	212 97 221	9.5	105	173	240	310	350		130			
118	116	113	110		104	98	97	97	95	105	173	240	143 310	345		130			
118	116	113	110	107	103	98	97	96	96	107	173	240	310	345		130			
117	114	111	109	106	102	98	97	96	96	107	172 170	240	307	340					
117	113	110	107	105	100	93	97	95	96	107		240	307	330					
116	113	110	107	104	100	78	97	222 96	95	183	170	240	305	320		129			
115	111	108	106	103	99	223 97	96	221 96	96	107	168	240	305	310					
113	110	107	103	100	232 98	97	221 96	95	96	107	165		300	300		126			
112	108	106	102	93	232 98	96	95	94	95	107	162 165	240	300	295		124			
109	107	105	100	99	98	96 105	95	94	95	170	165	237	295	290		120			
						195	1 4 5	190	1/5	165	152	140	133	125	120	118	115	112	110

#### B1. BASIC PACKAGE INPUT LISTING--CONTINUED

107	104	101	99	93	97	96 210	95 175	94 170				237 135			113	115	111	109	107
103	100	99	98	97	95	95	94 180	94 150				235			112	110	107	104	100
99	98	98	97	96	95	95	94	93	95	105	155	235		270	102		99	98	98
98	97	97	96	95	95	74	93	92 110	93 105	102		230 100	253 98	250 98	98	97	97	96	96
95	95	44	94	93	ý3	92	92	91	92		150	215	235	230	73	7 (	71	70	70
0.7	.5.7	170	165	160	155	135	115	100	99	98	98	97	97	96	96	95	95	94	94
93 180	93 175	92 170	92 165	91 160	91 155	91 135	90 125	90	91		130	175	205	190 95	94	93	92	91	91
90	90	90	89	39	\$9	88	88	89	90	100		98 140	96 170	140	74	73	92	41	91
		5	•	1.0	*	•		(20#		•	, 0,5	170	., .	, 40					
				270	260				150					150	150	150	150	150	150
150			150	_	148		145					200							
4 / 0			250			210	205 130					200		140	140	140	140	140	140
									125 147					130	120	128	127	126	125
					117		108			115			380	400	127	140	121	120	12)
	160		149		147			143							130	128	126	124	121
119	117	115	113	111	110	106	100	99	93	105	150	230	360	400					
	195	175			147				142						133	130	127	124	122
118	116				107		99	98				230							
110	111						149		147						132	129	126	123	120
115	110	114	112	200		103 175	99 150	93 148		146		237		400	135	171	127	12/	122
118	115	112	110		105	100	99	98		105		240		400	137	1 4 1	1 4 1	124	122
			,				175		149						134	130	127	124	122
118	116	113	110			100	98	97	96			240	315	400					
					210	500		160		147					134	130	127	124	123
118	116	113	110			99	98	97	96	105			310	390	47,	470	4 2 7	437	4 ~ =
112	114	113	110		210	200 200	185 98	165 97		148		144		370	134	130	127	124	123
110	115	113	110		210	500			150						134	130	127	124	122
118	116	113	110			98	97	97		105		240		360	1 😅 🕆		,	, 4 4	,
					210	500	185	165	150				143	138	134	130	127	124	122
118	116	113	110	107		98	97	96		107			310						
447	44/	4 4 4	400	407		200	135			150					134	130	127	124	121
117	114	111	109	100	210	98 200	97	96 175		107 150		240			133	110	127	1 2 7	120
117	113	110	107	105		98	97	96		107		240	307		133	130	161	123	120
					210	200	185	175		150					133	129	125	122	119
116	113	110	107	104		98	97	96		107		240		290					
					210	500	185	175	160	150	147	144	140	135	132	127	124	121	117
115	111	109	105	103											470	47/	4 2 2	440	* * /
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112	108	106	102	99	93	76	95	94				240							
					210	200	185	175	160						125	120	118	115	113
109	107	105	100	99	98	96	95	94				237							
4 7 7	104	101	20	3.0			180		160						120	113	115	112	110
107	104	101	99	93	97 200	<del>9</del> 6 190	95 130	94 170				237			118	115	111	100	107
103	100	99	98	97	76	95	94	94				235			ن ۱۱	, , ,	1 1 1	1 🔾 7	101
									150						112	110	107	104	100
99	93	98	97	96	95	9.5	94	93	9.5	105	155	235	270	270					
6.0	^ <b>~</b>	~ <del>"</del>	5.	~ ~					130						102	100	99	98	98
98	97	97	75	95	95	94	93	3.5	Q 3	102	155	230	253	270					

#### BL. BASIC PACKAGE INPUT LISTING-CONTINUED

550	215	210	200	150	105	100	100	100	100	102	102	100	98	98	98	97	97	96	9.6
95	95	94	94	93	93	92	92	91	92	99	150	215	235	260					
220	215	210	200	165	175	135	115	100	9.9	93	98	97	97	96	96	95	95	94	94
93	93	92	92	91	91	91	90	90	91	96	130	175	187	190					
500	183	167	150	145	140	135	125	115	107	100	99	98	95	95	94	93	92	91	91
90	90	90	39	89	89	88	38	हे 9	90	85	105	140	140	140		_	_	•	
8.64	005(	14		2		1 .	, 5												

#### B2. OUTPUT CONTROL LISTING

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C	1		
1	160	100	1
3	3	11	C
3	<del>-</del> 3	11	0
3	3	11	0